



DSA Structural Amendments under review are highlighted in GRAY

Template 23-24

2001 CBC - Chapter 23A
WOOD

Section - 2316A - DESIGN SPECIFICATIONS

Subsection(s) - 2316A.2 / 30. - 35.

30. Sec. 8.2. Delete and substitute as follows:

8.2.3 Allowable shear values for bolts used to connect a wood member to concrete or masonry are permitted to be determined as one half the tabulated double shear value for a wood member twice the thickness of the member attached to the concrete or masonry.

31. Sec. 12.2.3. Delete.

32. Sec. 12.3.7. Delete.

33. Sec. 12.4.1. Delete and substitute as follows:

12.4.1 For wood-to-wood joints, the spacing center to center of nails in the direction of stress shall not be less than the required penetration. Edge or end distances in the direction of stress shall not be less than one-half of the required penetration. All spacing and edge and end distances shall be such as to avoid splitting of the wood.

34. Sec. 13.2.1. Delete and substitute as follows:

13.2.1 Test for design values. Tests to determine design values for metal plate connectors in lateral withdrawal, net section shear and net section tension shall be conducted in accordance with the test and evaluation procedures in ANSI/TPI 1-1995. Design values determined in accordance with these test procedures shall be multiplied by all applicable adjustment factors (see Table 7.3.1) to obtain allowable design values.

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35. NDS Supplement Table 5A. Add combinations and design values as follows:

COMBINATION SYMBOL ⁴	SPECIES OUTER LAMINATION ⁵ /CORE LAMINATION ⁵	DESIGN VALUES IN POUNDS PER SQUARE INCH (psi)													
		BENDING ABOUT X-X AXIS (Loaded Perpendicular to Wide Faces of Laminations)						BENDING ABOUT Y-Y AXIS (Loaded Parallel to Wide Faces of Laminations)					AXIALLY LOADED		
		Bending		Compression Perpendicular to Grain		Shear Parallel to Grain ⁴ F_{vxz}	Modulus of Elasticity E_{xx}	Bending $E F_{yy}$	Compression Perpendicular to Grain (Side Faces) $F_{c,yy}$	Shear Parallel to Grain F_{vy}	Shear Parallel to Grain (For Members With Multiple Piece Laminations Which are not Edge glued) ¹⁵ F_{vy}	Modulus of Elasticity E_{yy}	Tension Parallel to Grain F_t	Compression Parallel to Grain F_c	Modulus of Elasticity E
		Tension Zone Stressed in Tension F_{txz}	Compression Zone Stressed in Tension ⁶ F_{bxz}	Tension Face ^{9,11} $F_{c,xx}$	Compression Face ^{9,10} $F_{c,xx}$										
VISUALLY GRADED SOUTHERN PINE															
26F-V1	SP/SP	2600	1300	650	650	200	1,800,000	1900	560	175	90	1,600,000	1150	1600	1,600,000
26F-V2	SP/SP	2600	1300	650	650	200	1,900,000	2200	650	175	90	1,800,000	1200	1650	1,800,000
26F-V3	SP/SP	2600	1300	650	650	200	1,900,000	2100	560	175	90	1,800,000	1150	1600	1,800,000
26F-V4 ⁸	SP/SP	2600	2600	650	650	200	1,900,000	2100	560	175	90	1,800,000	1150	1600	1,800,000
E-RATED SOUTHERN PINE															
28F-E1	SP/SP	2800	1400	650	650	200	2,000,000	1600	560	175	90	1,700,000	1300	1850	1,700,000
28F-E2 ⁸	SP/SP	2800	2800	650	650	200	2,000,000	1600	560	175	90	1,700,000	1300	1850	1,700,000
30F-E1 ¹⁵	SP/SP	3000	1500	650	650	200	2,000,000	1750	560	175	90	1,700,000	1250	1750	1,700,000
30F-E2 ^{8,15}	SP/SP	3000	3000	650	650	200	2,000,000	1750	560	175	90	1,700,000	1250	1750	1,700,000

¹⁵These combinations are only for nominal widths 6 inches and less, in accordance with AITC 117-93.

35